

Análise Não Linear de  
Estruturas Planas

CD - 1

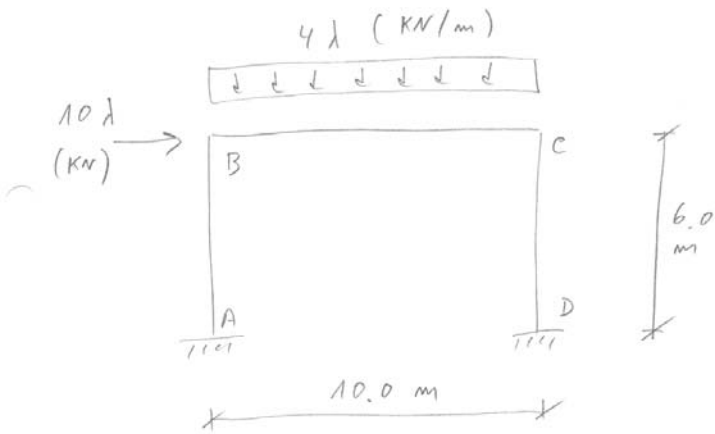
2005-12-08

Cargas distribuídas

Alvaro Azeredo

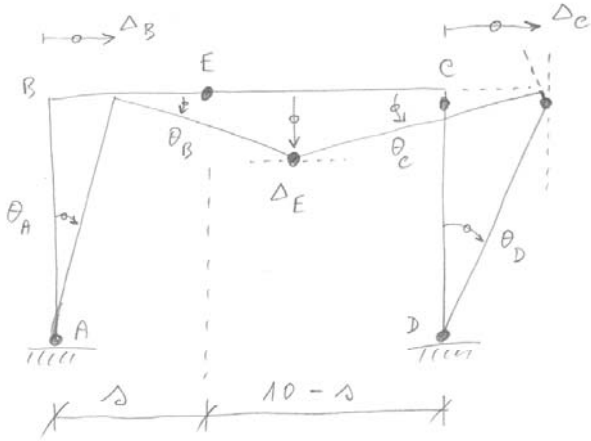
Exemplo

adaptado de (Ghali, Neville - 5: Edições - pág. 591)



Pilares:  
 $M_p^P = 120 \text{ kNm}$   
 Viga:  
 $M_v^V = 240 \text{ kNm}$

Mecanismo combinado:



$$\Delta_C = \Delta_B = 6 \theta_A$$

$$\theta_B = \theta_A$$

$$\theta_D = \theta_A$$

$$\Delta_E = \lambda \theta_B = \lambda \theta_A$$

$$\theta_C = \frac{\Delta_E}{10 - \lambda} = \frac{\lambda \theta_A}{10 - \lambda}$$

Cálculo de  $\lambda(s)$ :

CD-2

Trabalho externo = Trabalho interno

$$10\lambda \Delta_B + 4\lambda s \frac{\Delta E}{2} + 4\lambda(10-s) \frac{\Delta E}{2} =$$

$$= M_{\uparrow}^P \theta_A + M_{\uparrow}^V \theta_B + M_{\uparrow}^V \theta_c + M_{\uparrow}^P \theta_c + M_{\uparrow}^P \theta_D + M_{\uparrow}^P \theta_D$$

$$10\lambda 6\theta_A + 4\lambda s \frac{s\theta_A}{2} + 4\lambda(10-s) \frac{s\theta_A}{2} =$$

$$= 120\theta_A + 240\theta_A + 240 \frac{s\theta_A}{10-s} + 120 \frac{s\theta_A}{10-s} + 120\theta_A + 120\theta_A$$

$$60\lambda + 2\lambda s^2 + 20\lambda s - 2\lambda s^2 = 600 + \frac{360s}{10-s}$$

$$(60 + 20s)\lambda = \frac{6000 - 240s}{10-s}$$

$$\lambda = \frac{6000 - 240s}{600 - 60s + 200s - 20s^2} = \frac{6000 - 240s}{600 + 140s - 20s^2}$$

$$\left(\frac{u}{v}\right)' = \frac{u'v - uv'}{v^2}$$

$$\frac{d\lambda}{ds} = \frac{(-240)(600 + 140s - 20s^2) - (6000 - 240s)(140 - 40s)}{(600 + 140s - 20s^2)^2} = 0$$

$$s^* = 4.506098 \text{ m}$$

$$\lambda^* = 5.963631 \rightarrow \text{valor m\u00ednimo de } \lambda(s)$$

CD-3

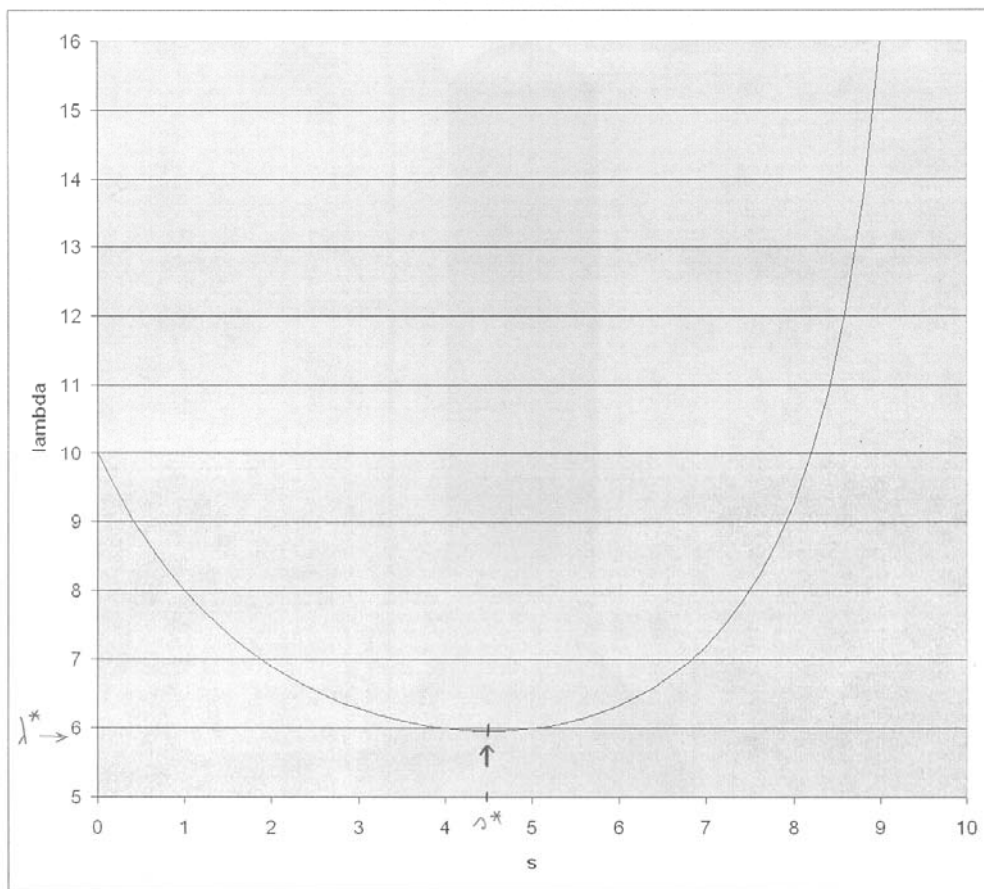
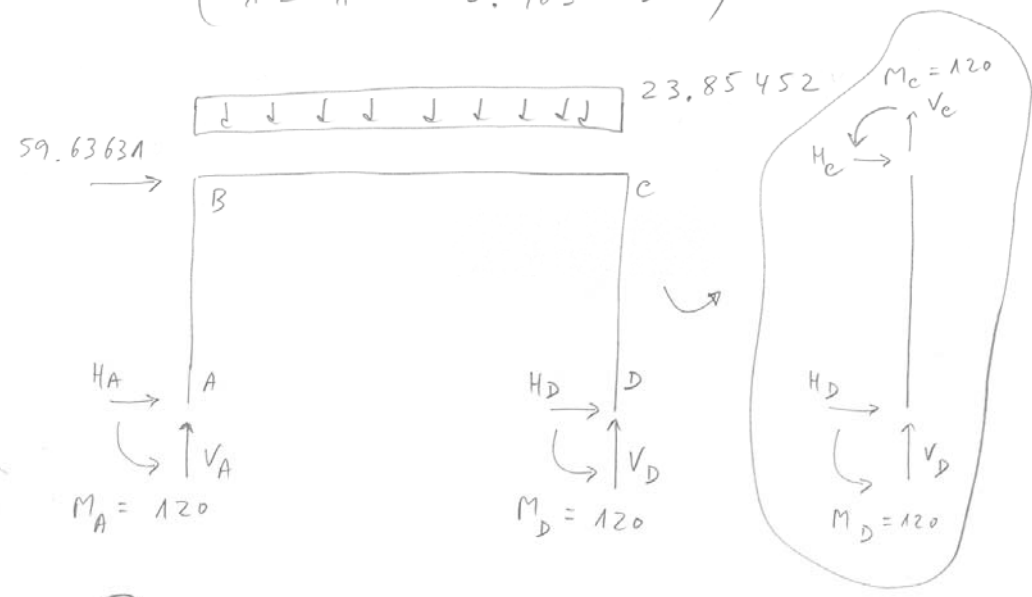


grafico - s - lambda . x l s

Diagrama de momentos final  
 (  $l = l^* = 5.963\ 631$  )

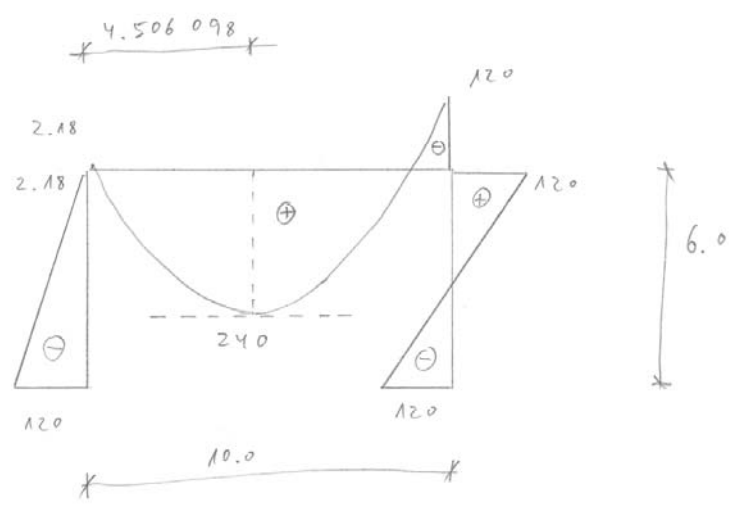
CD-4



$\sum \oplus M_c = 0 \Rightarrow 2 \times 120 + 6 H_D = 0 \Rightarrow H_D = -40\ \text{KN}$   
 (Barra CD)

$\sum \oplus F_H = 0 \Rightarrow H_A + H_D + 59.63631 = 0 \Rightarrow H_A = -19.63631\ \text{KN}$   
 (total)

$M_B = -120 - 6 H_A = -2.18214\ \text{KNm}$



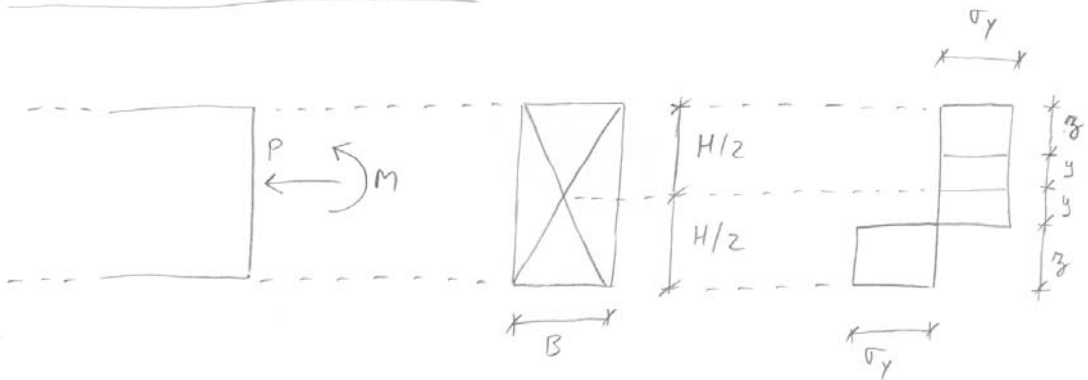
# Análise Não Linear de Pórticos Planos

EEA - 1

2005-12-08

Alvaro Azavedo

Efeito do esforço axial



$$z = \frac{H}{2} - y$$

$$P = 2 y B \sigma_y \Rightarrow \boxed{P = 2 \sigma_y B y}$$

$$M = 2 \left[ z B \sigma_y \left( y + \frac{z}{2} \right) \right]$$

$$M = 2 \left[ \left( \frac{H}{2} - y \right) B \sigma_y \left( y + \frac{H}{4} - \frac{y}{2} \right) \right]$$

$$M = 2 B \sigma_y \left( \frac{H y}{2} + \frac{H^2}{8} - \frac{H y}{4} - y^2 - \frac{H y}{4} + \frac{y^2}{2} \right)$$

$$M = 2 B \sigma_y \left( \frac{H^2}{8} - \frac{y^2}{2} \right)$$

$$\boxed{M = \sigma_y B \left( \frac{H^2}{4} - y^2 \right)}$$

$$\begin{cases} P = 2 \sigma_y B y \\ M = \sigma_y B \left( \frac{H^2}{4} - y^2 \right) \end{cases}$$

$$y = 0 \Rightarrow P = 0 \quad ; \quad M = \sigma_y \frac{BH^2}{4}$$

$$y = \frac{H}{2} \Rightarrow P = \sigma_y BH \quad ; \quad M = 0$$

$$\bar{M} = \sigma_y \frac{BH^2}{4} \quad ; \quad \bar{P} = \sigma_y BH$$

$$M = \sigma_y B \left( \frac{H^2}{4} - y^2 \right) \quad ; \quad P = 2 \sigma_y B y$$

$$\frac{M}{\bar{M}} = \frac{\frac{H^2}{4} - y^2}{\frac{H^2}{4}} = 1 - \frac{4y^2}{H^2} = 1 - \left( \frac{2y}{H} \right)^2$$

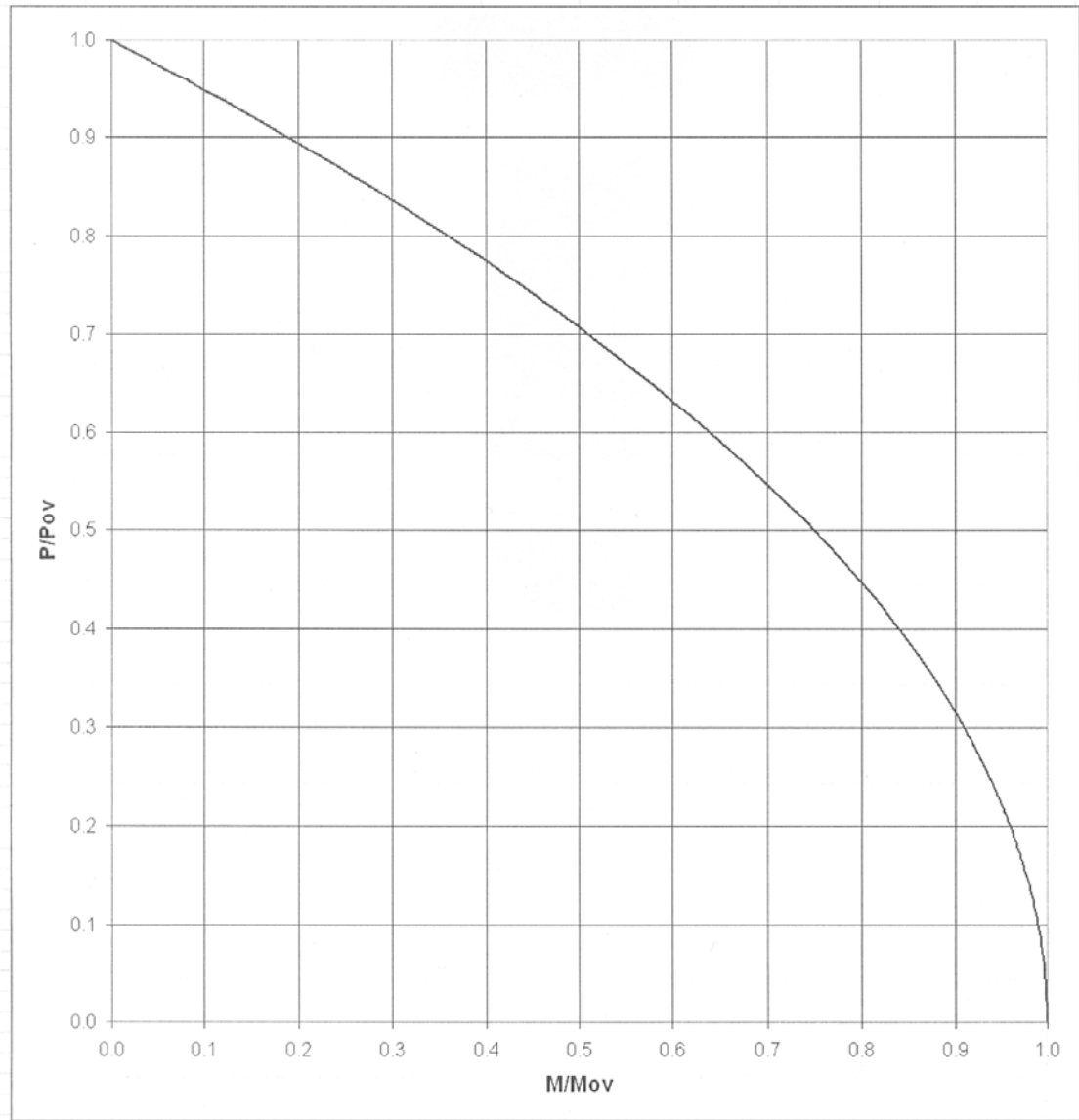
$$\frac{P}{\bar{P}} = \frac{2y}{H}$$

$$\frac{M}{\bar{M}} = 1 - \left( \frac{P}{\bar{P}} \right)^2$$

$$\frac{P}{\bar{P}} = \sqrt{1 - \frac{M}{\bar{M}}}$$

EEA-3

File: grafico\_M\_P.xls  
Date: 2005-12-12



Optimização de Pontes  
Planos em Regime Não Linear

OPP-1

2005-12-08

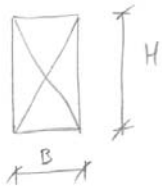
Alvaro Azeredo

B. G. Neal

The Plastic Methods of Structural Analysis

3<sup>rd</sup> Edition, Capítulo 7

Chapman and Hall, 1977



$$B = cH \quad (\text{onde } c \text{ fixo})$$

$$A = BH = cH^2$$

$$M_p = \sigma_y \frac{BH^2}{4} = \sigma_y \frac{cH^3}{4} = \frac{\sigma_y c}{4} H^3$$

$$H^3 = \frac{4M_p}{\sigma_y c} \Rightarrow H^2 = \left( \frac{4}{\sigma_y c} \right)^{2/3} M_p^{2/3}$$

$$A = c \left( \frac{4}{\sigma_y c} \right)^{2/3} M_p^{2/3} = \left( \frac{4}{\sigma_y} \right)^{2/3} \frac{c}{c^{2/3}} M_p^{2/3} = \left( \frac{4}{\sigma_y} \right)^{2/3} c^{1/3} M_p^{2/3}$$

$\rho \rightarrow$  peso da barra por unidade de comprimento

$$\rho = \gamma A$$

$$\rho = \gamma \left( \frac{4}{\sigma_y} \right)^{2/3} c^{1/3} M_p^{2/3}$$

$P \rightarrow$  peso da estrutura (barras de secção constante)

$$P = \sum_{i=1}^{NB} \rho_i L_i = \sum_{i=1}^{NB} \left[ \gamma \left( \frac{4}{\sigma_y} \right)^{2/3} c^{1/3} M_{pi}^{2/3} L_i \right]$$



$$P = \gamma \left( \frac{y}{\sigma_y} \right)^{2/3} c^{1/3} \sum_{i=1}^{NB} \left[ M_{\uparrow i}^{2/3} L_i \right]$$

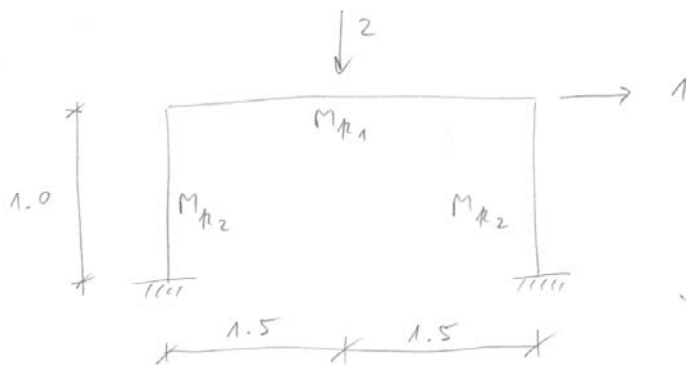
Minimizar  $P$  é o mesmo que minimizar

$$P' = \sum_{i=1}^{NB} \left[ L_i M_{\uparrow i}^{2/3} \right]$$

De um modo simplificado pode-se considerar

$$P' \approx \sum_{i=1}^{NB} \left[ L_i M_{\uparrow i} \right]$$

Exemplo:

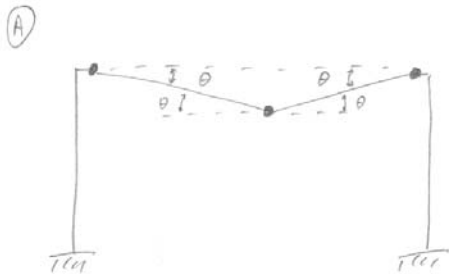


Calcular a solução  $(M_{\uparrow 1}^*, M_{\uparrow 2}^*)$  a qual corresponde um peso total mínimo

$$\begin{cases} M_{\uparrow 1} \rightarrow r_1 \\ M_{\uparrow 2} \rightarrow r_2 \end{cases}$$

$$M_{p1} \leq M_{p2} \quad (v_1 \leq v_2)$$

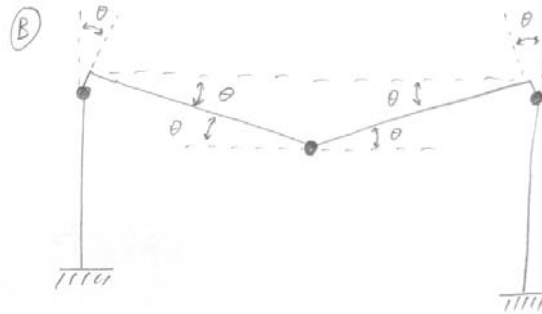
$$M_{p1} \geq M_{p2} \quad (v_1 \geq v_2) \quad \text{OPP-3}$$



$$2 \times 1.5 \theta = 4 M_{p1} \theta$$

$$4v_1 - 3 = 0$$

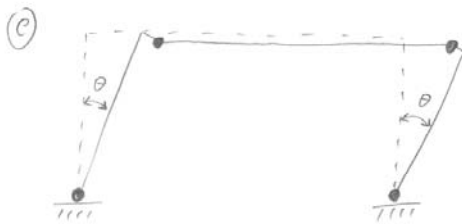
$$v_1 \geq 3/4$$



$$2 \times 1.5 \theta = 2 M_{p1} \theta + 2 M_{p2} \theta$$

$$2v_1 + 2v_2 - 3 = 0$$

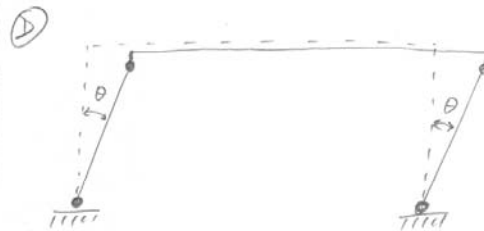
$$v_2 \geq -v_1 + 3/2$$



$$1 \times 1 \times \theta = 2 M_{p1} \theta + 2 M_{p2} \theta$$

$$2v_1 + 2v_2 - 1 = 0$$

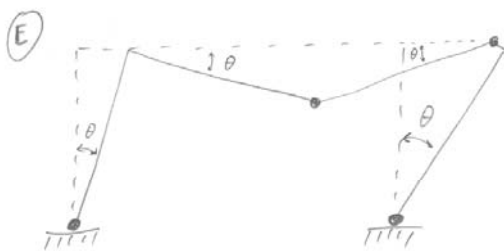
$$v_2 \geq -v_1 + 1/2$$



$$1 \times 1 \times \theta = 4 M_{p2} \theta$$

$$4v_2 - 1 = 0$$

$$v_2 \geq 1/4$$



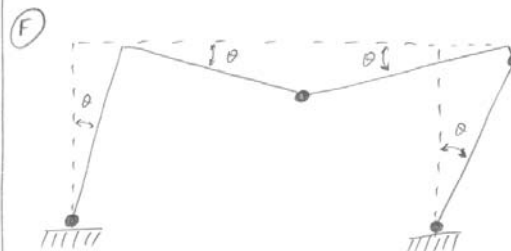
$$2 \times 1.5 \theta + 1 \times 1 \times \theta =$$

$$= M_{p2} \theta + 2 M_{p1} \theta + 2 M_{p1} \theta + M_{p2} \theta$$

$$4 = 4v_1 + 2v_2$$

$$2v_1 + v_2 - 2 = 0$$

$$v_2 \geq -2v_1 + 2$$



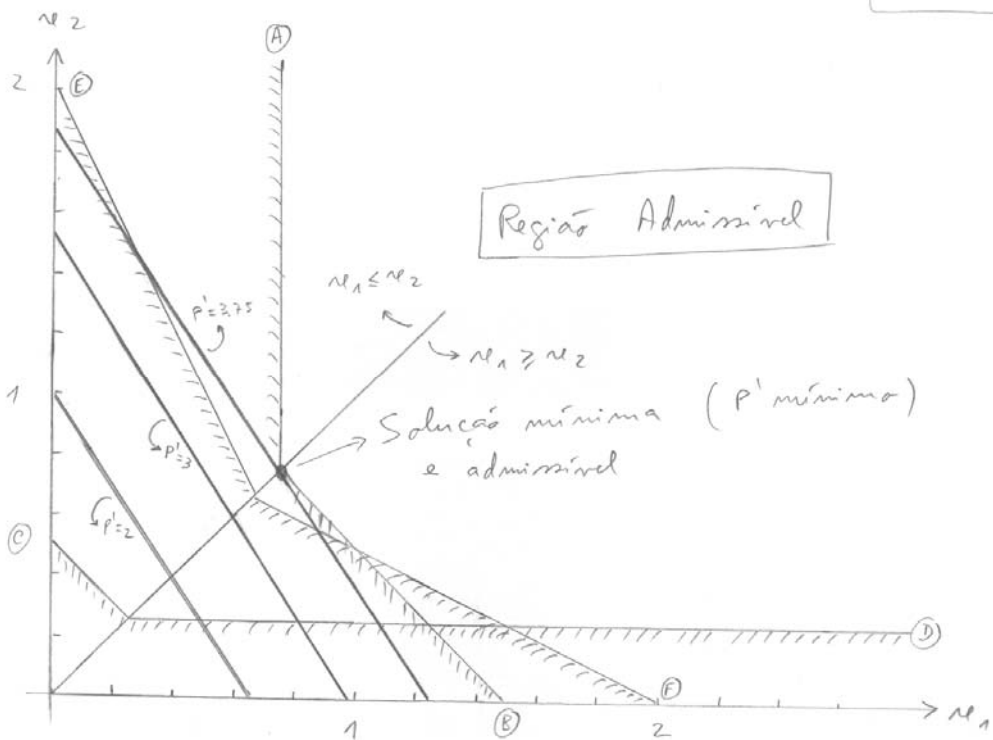
$$2 \times 1.5 \theta + 1 \times 1 \times \theta =$$

$$= M_{p2} \theta + 2 M_{p1} \theta + 2 M_{p2} \theta + M_{p2} \theta$$

$$4 = 2v_1 + 4v_2$$

$$v_1 + 2v_2 - 2 = 0$$

$$v_2 \geq -\frac{1}{2}v_1 + 1$$



Minimizar:

$$P' = 3x_1 + 2x_2$$

$$\begin{cases} 3x_1 + 2x_2 = 2 \Rightarrow x_2 = -\frac{3}{2}x_1 + 1 \\ 3x_1 + 2x_2 = 3 \Rightarrow x_2 = -\frac{3}{2}x_1 + \frac{3}{2} \\ 3x_1 + 2x_2 = 3.75 \Rightarrow x_2 = -\frac{3}{2}x_1 + 1.875 \end{cases}$$

$$\text{Solução} = (x_1^*, x_2^*) = (0.75, 0.75) \searrow$$

Interseção das retas  
 (A) e (D)

$$\begin{cases} M_{x_1}^* = 0.75 \\ M_{x_2}^* = 0.75 \end{cases}$$